

COMPACT CORE BEARING

Unreinforced heavy-duty elastomeric bearing Thermal separation in structural steelwork

Bearing design formulae

Product description

The Calenberg Compact Core Bearing is an unreinforced heavy-duty elastomeric bearing with a smooth surface. The red-brown colour of the material clearly identifies the product.

How to specify

Supply Calenberg Compact Core Bearing, unreinforced homogeneous elastomeric bearing in accordance with DIN 4141 Part 3, bearing class 2, through-coloured red-brown with smooth surface, loadable depending on format up to an average compressive stress of 30 N/mm², National Technical Approval No. P-852.0448.

a) Standard Installation

b) Embedded in polystyrene or Ciflamon fire-proofing plate

Supplier:

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Shape factor

Deflection 2

Design table 1

Compact core bearing, **15 and 20 mm** thick

Design table 2

Design example

Fig. 2: Influence of elastomer thickness on structural behaviour, determined using finite element analysis

Design of end plate connection with intermediate elastomeric bearing

(after: Dr.-Ing. L. Nasdala; Dr.-Ing. B. Hohn, R. Rühl Institute for Structural Analysis University of Hanover, Faculty for Construction Engineering and Geodetics in "Der Bauingenieur" – 11/2005)

Cold bridges are created in buildings when steel beams penetrate the building envelope. In addition to the associated heat loss, this frequently leads to the formation of mould due to condensation forming on the indoor wall surfaces. If thermal insulation cannot be attached to the external steelwork components, e.g. for architectural reasons, then thermal separation of the internal and external structure is required. Thermal separation can be provided by elastomeric bearings with a thermal conductivity of $\lambda \approx 0.2$ W/ (mK), which, although five to ten times higher than conventional insulation materials such as glass wool or polystyrene, is more than two hundred times lower than structural steel. The most important advantage over traditional insulation materials is the bearing's higher loadbearing capacity.

Figure 1 shows the traditional end plate butt joint. The details of the example design are given on the following pages.

Fig. 3: Determination of the compressive stress in the elastomer

The end plates of the IPE 300 beam butt joint are made from S 235 steel. A height of $h_e = 320$ mm, a width of $b_e = 130$ mm and a thickness of $t_e = 10$ mm were selected for the elastomer. The fasteners are 4 x M20 bolts strength class 10.9 with a 1 mm tolerance.

The design of the joint without the intermediate elastomer layer in accordance with EC 3 will not be discussed any further here.

As elastomers are almost incompressible, they bulge out at the sides under load. Therefore – for the same material properties – thick elastomer plates cannot carry as much load as thin ones. Using the Shape Factor S, and taking into account the elastomer dimensions and the number

and diameter of the bolts, the allowable average compressive stress can be determined. It is defined as the ratio of effective structural bearing surface A_m to the associated side area As.

$$
S = \frac{A_m}{A_s}
$$
 [1]

As the accurate distribution of the compressive stress is unknown, a linear stress distribution is assumed initially for the bearing design. As shown in Figure 3, the stress distribution is converted using the equilibrium of normal forces and moments into an average stress σ_m and an effective height hm. The number of bolt holes to be taken into account depends on the value of this height h_m .

For a rectangular bearing plate with 2 or 4 holes, the Shape Factor is:

$$
S = \frac{h_m \cdot b_e - \pi d^2/2}{2 \cdot t_e \cdot (h_m + b_e + \pi d)} \quad \text{if} \quad h_m \le \frac{2}{3} \, h_e
$$
\n
$$
S = \frac{h_m \cdot b_e - \pi d^2}{2 \cdot t_e \cdot (h_m + b_e + 2 \, \pi d)} \quad \text{if} \quad h_m > \frac{2}{3} \, h_e
$$

The allowable average compressive stress in the elastomer of the Core Compact Bearing is calculated using:

allw.
$$
\sigma_m = \frac{S^2 + S + 1}{0.70} \le 30 \text{ N/mm}^2
$$
 [4]

Linear stress distribution

If the holes are disregarded and a linear distribution assumed, the stresses are calculated according to the equation:

$$
\sigma(z) = \frac{N - 4 \text{ Fs}}{b_e h_e} + \frac{12 \text{ my}}{b_e h_e^3} z
$$
 [5]

with edge stresses $\sigma_0 = \sigma$ (-h_e/2) and $\sigma_u = \sigma (+ h_e/2)$

If computer analyses indicate that tensile stresses occur, they result in a bolt tensile force F.

Design example

Design example

At point

$$
z_0 = \frac{4 F_s - N}{12 M_y} h_e^2 \in \left[-\frac{h_e}{2}; +\frac{h_e}{2} \right]
$$
 [6]

a large moment My results in a sign change, $\sigma(z_0) = 0$.

Compressive stress only

For
\n
$$
z_0 \in \left[-\frac{h_e}{2}; +\frac{h_e}{2}\right] \text{ and } 4 \text{ F}_s > \text{N it follows:}
$$
\n
$$
h_m = h_e + \frac{2 \text{ M}_y}{\text{N} - 4 \text{ F}_s} \text{ and}
$$
\n
$$
\sigma_m = \frac{(\text{N} - 4 \text{ F}_s)^2}{b_e[h_e(\text{N} - 4 \text{ F}_s) + 2 \text{ M}_y]}
$$
\n[8]

Compressive & tensile stresses

For $z_0 \in \left[-\frac{h_e}{2}; +\frac{h_e}{2}\right]$ and $M_y > 0$ the bolt

tensile forces are calculated as:

$$
F=\frac{N-4~F_s}{h_e}\left(\frac{h_e}{2}-z_o\right)+\frac{6~M_y}{h_e{}^3}\left(\frac{h_{e^2}}{4}-z_{o^2}\right)~\textbf{[9]}
$$

and the following apply

$$
h_m = h_e + \frac{2 M_y - F e_2}{N - 4 F_s - F} \text{ and } [10]
$$

$$
\sigma_{m} = \frac{(N - 4F_{s} - F)^{2}}{b_{e}[h_{e}(N - 4F_{s} - F) + 2M_{y} - F \cdot e_{2}]} \qquad [11]
$$

Example calculation:

Bending moment $M_y = 30$ kNm Normal force $N = -20$ kN Bolt prestress force $F_s = 80$ kN/bolt

 $z_{\circ} = \frac{4 \cdot 80 - (-20)}{12 \cdot 30} \cdot 0,32^{2} = 0,097 \,\text{m}$ using [6]

As $M_v > 0$ the bolt tensile force is calculated using **[9]**

 $F = \frac{(-20) - 4 \cdot 80}{0,32} \cdot \frac{(0,32)}{2} - 0,097 + \frac{6 \cdot 30}{0,32^3} \cdot \frac{(0,32^2)}{4} - 0,097^2$ $F = 22$ kN

and an effective height hm using **[10]**

 $h_m = 0,32 + \frac{2 \cdot 30 - 22 \cdot 0,21}{-20 - 4 \cdot 80 - 22} = 0,167$ m

The average compressive stress using **[11]** $\sigma_{\rm m} = \frac{(-20 - 4.80 - 22)^2}{10^3 \cdot 0,13[0,32(-20 - 4.80 - 22) + 2.30 - 22.0,21]}$

 $σ_m = 16,67$ N/mm²

From h_m = 0,167 m $<$ $\frac{2}{3}$ 0,32 = 0,21 m

the shape factor is calculated using **[2]**

$$
S = \frac{167 \cdot 130 - \pi \cdot 21/2}{2 \cdot 10 \cdot (167 + 130 + \pi \cdot 21)} = 2.9
$$

The allowable bearing load in accordance with **[4]** is

allow. $\sigma_{\text{m}} = \frac{2.9^2 + 2.9 + 1}{0.70} = 17.58 \text{ N/mm}^2 \le 30 \text{ N/mm}^2$

With the following result

Actual $\sigma_m = 16{,}67 \text{ N/mm}^2 \leq \text{Allow.} \ \sigma_m = 17{,}58 \text{ N/mm}^2$ compliance with the requirements is confirmed.

Characteristics

Due to its high material hardness, the Compact Core Bearing – in contrast to ordinary, softer elastomeric bearings – deforms very little under load. In practice this means:

- The high stiffness of the bearing means it is not suitable for accommodating shear deformations and rotations.
- Transverse deformation is extremely low thanks to the bearing's excellent shape stability.
- Moments are transferred without large deformations.
- Due to its low deformation and high thermal resistivity, the bearing is particularly suitable for use as a thermal separation layer in end plate butt joints in structural steelwork.

Use and areas of application

Compact Core Bearings are used in all fields of metal construction to provide thermal separation, such as in the installation of building facades, solar energy equipment on roofs or the connection of balconies and canopies to the main loadbearing structure.

Material

Elastomeric material based on butadieneacrylonitrile rubber, colour red-brown (common short name: NBR (nitrile rubber)

Compact Core Bearings are resistant to oil, grease and fuel; they are also resistant to abrasion and wear.

Delivery forms

Calenberg Compact Core Bearings are supplied ready-made in the shape and dimensions required for each project. (Figure 4)

Holes, cut-outs, slots etc. can be provided to allow bolts or dowels to pass through the bearings.

Dimensions

- Bearing thicknesses: 5, 10, 15, 20 mm
- Maximum cut dimensions: 1200 mm x 1200 mm

Characteristics

Test certificate

Test certificate, proof of suitability

- National Technical Approval No. P-852.0448 Compression, shear and creep tests on building bearing hard spring plate "NBR Compact Core Bearing"; Testing Institute for Mechanical Engineering Materials and Plastics, Technical University of Hanover, 2003
- Fire Safety Assessment No. 3799/7357-AR; Assessment of Calenberg elastomeric bearings regarding classification into the fire resistance class F 90 or F 120 according to DIN 4102 part 2 (issued 9/1977); Accredited Material Testing Authority for Civil Engineering at the Institute for Construction Materials, Reinforced Concrete Construction and Fire Protection, Technical University, Braunschweig; March 2005.

Fire behaviour

Fire Safety Report No. 3799/7357-AR by the Technical University (TU) of Braunschweig shall be determinant for elastomeric bearings installed in situations where fire safety has to be taken into account. The report describes minimum dimensions and other measures that fulfil the requirements of DIN 4102-2: Fire Behaviour of Building Materials and Building Components, 1977-09

The contents of the publication in the result of many years of research an experience gained in application technology. All
information is given in good fairly; it does not represent a
guarantiee with respect to characteristics an does not exempt
the user from testing the suitability personal liability of or legal representatives and employed in performing our obligations.

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Standard cut-outs

